

Traffic Signal Warrant Study

SH 5 (N Powell Pkwy) at Rosamond Pkwy

Anna, Texas



MALDONADO - BURKETT

ENGINEERS | CONTRACTORS

TBPE# 10258



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Prepared for

City of Anna
3223 N. Powell Parkway
Anna, Texas 75409

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Executive Summary

The City of Anna has requested Maldonado-Burkett (M-B) to perform a traffic signal warrant analysis at the intersection of SH 5 (N Powell Pkwy) and Rosamond Pkwy, and to make a recommendation on whether a traffic signal should be considered at the study intersection. M-B engineers have analyzed all nine signal warrants from the Texas MUTCD (Revision 2 in 2014) using the collected traffic volume and historical crash data.

The analysis results are shown as below:

Warrant	Description	Analysis Result
1A	Eight Hour Volume – Minimum Vehicular Volume (70% & 56%)	Warrant Not Met
1B	Eight Hour Volume – Interruption of Continuous Traffic (70% & 56%)	Warrant Met
2	Four Hour Vehicular Volume	Warrant Met
3	Peak Hour	Warrant Met
4	Pedestrian Volume	Warrant Not Met
5	School Crossing	Warrant Not Met
6	Coordinated Signal System	Warrant Not Met
7	Crash Experience	Warrant Met
8	Roadway Network	Not Applicable
9	Intersection Near a RR Grade Crossing	Warrant Not Met

The warrants that were met are Warrant 1B, 2, 3, and 7. The traffic volumes present at this intersection are above the required volumes to meet Warrants 1B, 2, and 3. After reviewing crash data at this intersection, there have been no incapacitating injury or fatality crashes in the past year. Of the crashes reported, there are a total of seven reported crashes within the last year of the study intersection with one of them being a major injury case and the other six being minor injury and/or property damage crashes. Therefore, Warrant 7 is met.

Bases on the results of the study, the installation of a traffic signal is recommended at this time.



1. Introduction

The City of Anna has requested Maldonado-Burkett (M-B) to perform a traffic signal warrant analysis at the intersection of SH 5 (N Powell Pkwy) and Rosamond Pkwy, and to make a recommendation on whether a traffic signal should be considered at the study intersection. SH 5, which is considered as the “major street” in this study, is a 2-lane, 2-way undivided roadway with 12-foot lanes and two 5-foot shoulders. The posted speed limit on SH 5 in the study area is 55 MPH. Rosamond Pkwy, which is considered as the “minor street” in this study, is a 2-lane, 2-way undivided roadway with a lane width of approximately 10 feet on the west leg and 16 feet on the east leg. The posted speed limit on Rosamond Pkwy is 30 MPH. Figure 1 shows the location of the study intersection.

Figure 1. Study Intersection Location



The analysis was performed in accordance with the Texas Manual on Uniform Traffic Control Devices (Texas MUTCD) 2014 Revision, Chapter 4C. The goal of this project is to analyze traffic volume data and crash records to determine if the traffic signal warrants are met in accordance with the requirements listed in the Texas MUTCD.



2. Data Collection

A 24-hour turning movement volume was collected on October 24th, 2023 at the study intersection by Gram Traffic North Texas. The traffic volume dataset, which is included in **Appendix B** includes the traffic modes of motor vehicle, bicyclists, and pedestrians.

Crash history of the study intersection for the past 12 months dated back from the analyzing date was obtained through TxDOT's Crash Records Information System (CRIS), and is included in **Appendix C**.

3. Study Approach

The Texas MUTCD contains nine traffic signal warrants, which address a variety of intersection conditions such as vehicular volume, pedestrian volume, crashes, progression, and delay; and establish minimum criteria for consideration of traffic signal installation. The investigation of the need for a traffic control signal shall include an analysis of the applicable factors contained in the traffic signal warrants and other factors related to existing operation and safety at the study location. The nine warrants are listed below:

- Warrant 1, Eight-Hour Vehicular Volume
- Warrant 2, Four-Hour Vehicular Volume
- Warrant 3, Peak Hour
- Warrant 4, Pedestrian Volume
- Warrant 5, School Crossing
- Warrant 6, Coordinated Signal System
- Warrant 7, Crash Experience
- Warrant 8, Roadway Network
- Warrant 9, Intersection near a Grade Crossing

Section 3.1 through Section 3.9 in this chapter show the criteria of each of the nine warrants, and present the corresponding analyses results in general. Detailed analyses for each warrant are included in **Appendix A**.

3.1. Warrant 1, Eight-Hour Vehicular Volume

The purpose of Warrant 1 is to determine whether intersecting traffic from the minor approaches are the main reason to consider installing a traffic signal. This warrant includes two conditions:

- Condition A – the Minimum Vehicular Volume, is intended for application at locations where a large volume of intersecting traffic is the principal reason to consider installing a traffic control signal;
- Condition B – the Interruption of Continuous Traffic is intended for application at locations where Condition A is not satisfied and where the traffic volume on a major street is so heavy that traffic on a minor intersecting street suffers excessive delay or conflict in entering or crossing the major street.



Table 4C-1 from Texas MUTCD that shows the minimum requirements of eight-hour traffic volumes for both conditions is presented as below:

Table 1. MUTCD Table 4C-1: Warrant 1, Eight-Hour Vehicular Volume

Condition A—Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (Total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100%a	80%b	70%c	56%d	100%a	80%b	70%c	56%d
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B—Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (Total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100%a	80%b	70%c	56%d	100%a	80%b	70%c	56%d
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

- a - Basic minimum hourly volume;
- b - Used for combination of Conditions A and B after adequate trial of other remedial measures;
- c - May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000;
- d - May be used for combination of Conditions A and B after adequate trial of other remedial measures when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000.

Per the Texas MUTCD standards, the need for a traffic control signal shall be considered if an engineering study finds that one of the following conditions exist for each of any 8 hours of an average day:

- A. The vehicles per hour given in both of the 100 percent columns of Condition A in Table 4C-1 exist on the major-street and the higher-volume minor-street approaches, respectively, to the intersection; or
- B. The vehicles per hour given in both of the 100 percent columns of Condition B in Table 4C-1 exist on the major-street and the higher-volume minor-street approaches, respectively, to intersection.



The warrant is also satisfied if both Conditions A and B are met at the 80% level for any 8 hours of an average day.

If the posted or statutory speed limit or the 85th-percentile speed on the major street exceeds 40 mph, or if the intersection lies within the built-up area of an isolated community having a population of less than 10,000, the traffic volumes in the 70 percent columns in Table 4C-1 may be used in place of the 100 percent columns.

The posted speed limit on SH 5 adjacent to the study intersection is 55 MPH. Therefore, by comparing the traffic volumes of the eight highest hours of the study intersection with the volumes in the 70% and 56% columns in Table 4C-1, it is determined that **Warrant 1 is met** for the study intersection. The 24-hour traffic volume on October 24, 2023 is presented in Table 2 as below. Detailed analyses process is included in **Appendix A**.

Table 2. Warrant 1 Analysis

Time	Major St.	Higher Volume Minor St.	Meet Condition A (70%)?	Meet Condition B (70%)?	Meet Condition A & B (56%)?
12:00 AM	24	8	No	No	No
1:00 AM	10	1	No	No	No
2:00 AM	23	0	No	No	No
3:00 AM	29	1	No	No	No
4:00 AM	66	9	No	No	No
5:00 AM	189	28	No	No	No
6:00 AM	463	85	No	No	No
7:00 AM	726	165	Yes	Yes	Yes
8:00 AM	692	116	Yes	Yes	Yes
9:00 AM	483	50	No	No	No
10:00 AM	439	64	No	No	No
11:00 AM	543	56	No	Yes	No
12:00 PM	585	73	No	Yes	No
1:00 PM	507	70	No	No	No
2:00 PM	550	83	No	Yes	No
3:00 PM	694	117	Yes	Yes	Yes
4:00 PM	1059	118	Yes	Yes	Yes
5:00 PM	1012	168	Yes	Yes	Yes
6:00 PM	770	161	Yes	Yes	Yes
7:00 PM	497	86	No	No	Yes
8:00 PM	322	50	No	No	No
9:00 PM	283	61	No	No	No
10:00 PM	134	13	No	No	No
11:00 PM	59	8	No	No	No
No. of Hours Meeting Condition:			6	9	7
No. Hours Required:			8	8	8
Meet Warrant 1:			No	Yes	No



3.2. Warrant 2, Four-Hour Vehicular Volume

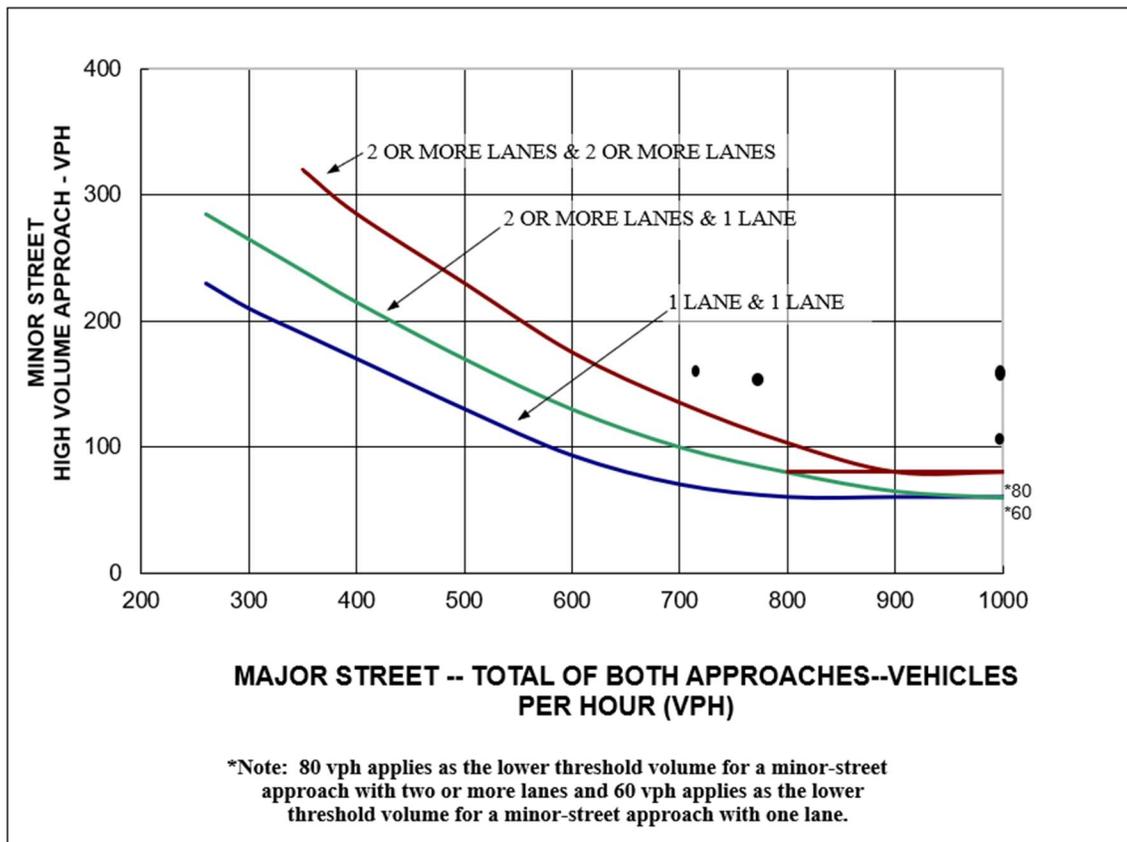
Similar to Warrant 1, Warrant 2, Four-Hour Vehicular Volume is also intended to be applied where the volume of intersecting traffic is the principal reason to consider installing a traffic control signal.

Per the Texas MUTCD standards, the need for a traffic control signal shall be considered if an engineering study finds that, for each of any 4 hours of an average day, the plotted points representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor-street approach (one direction only) all fall above the applicable curve in Figure 4C-1 for the existing combination of approach lanes. On the minor street, the higher volume shall not be required to be on the same approach during each of these 4 hours.

If the posted or statutory speed limit or the 85th-percentile speed on the major street exceeds 40 mph, or if the intersection lies within the built-up area of an isolated community having a population of less than 10,000, Figure 4C-2 may be used in place of Figure 4C-1.

As the posted speed limit on SH 5 is 55 MPH, Figure 4C-2 from Texas MUTCD that shows the minimum requirements of four-hour traffic volumes is used in the analysis and is presented on next page.

Figure 2. MUTCD Figure 4C-2 Warrant 2, Four-Hour Vehicular Volume (70% Factor)





By comparing the traffic volumes of the four highest hours of the study intersection with the volumes in Figure 4C-2, it is determined that Warrant 2 is met at the study intersection. Detailed analysis process is included in **Appendix A**.

Warrant 2 is met for the study intersection.

3.3. Warrant 3, Peak Hour

The Peak Hour signal warrant is intended for use at a location where traffic conditions are such that for a minimum of 1 hour of an average day, the minor-street traffic suffers undue delay when entering or crossing the major street. This signal warrant shall be applied only in unusual cases, such as office complexes, manufacturing plants, industrial complexes, or high-occupancy vehicle facilities that attract or discharge large numbers of vehicles over a short time.

A traffic signal shall be considered if an engineering study finds that the criteria in either of the following two categories are met:

A. If all three of the following conditions exist for the same 1 hour (any four consecutive 15-minute periods) of an average day:

1. The total stopped time delay experienced by the traffic on one minor-street approach (one direction only) controlled by a STOP sign equals or exceeds: 4 vehicle-hours for a one-lane approach or 5 vehicle-hours for a two-lane approach; and
2. The volume on the same minor-street approach (one direction only) equals or exceeds 100 vehicles per hour for one moving lane of traffic or 150 vehicles per hour for two moving lanes; and
3. The total entering volume serviced during the hour equals or exceeds 650 vehicles per hour for intersections with three approaches or 800 vehicles per hour for intersections with four or more approaches.

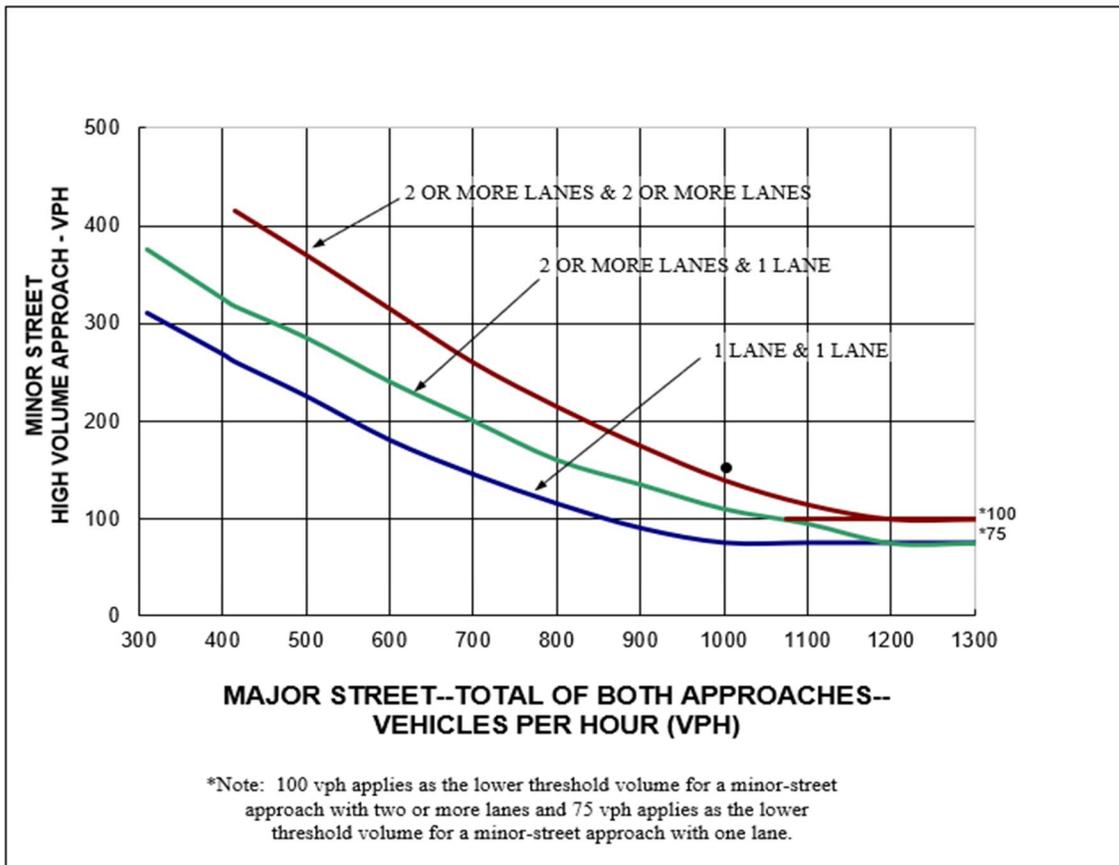
B. The plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor-street approach (one direction only) for 1 hour (any four consecutive 15-minute periods) of an average day falls above the applicable curve in Figure 4C-3 for the existing combination of approach lanes.

If the posted or statutory speed limit or the 85th-percentile speed on the major street exceeds 40 mph, or if the intersection lies within the built-up area of an isolated community having a population of less than 10,000, Figure 4C-4 may be used in place of Figure 4C-3 to evaluate the criteria in the second category of the Standard.

Figure 4C-4 presented on next page is used in this study as the posted speed limit on SH 5 is 55 MPH.



Figure 3. MUTCD Figure 4C-4 Warrant 3, Peak Hour (70% Factor)



Through applying the highest hourly volume of the study intersection to Figure 4C-4, it can be seen that Category B is met for this Warrant. Per the Texas MUTCD, if either of the two categories is met, a traffic control signal shall be considered at the study location.

Per the collected traffic volume data, the AM and PM peak hours at this intersection was identified as 7:00-8:00 in the morning and 4:00-5:00 in the afternoon. Considering the fact that the Anna High School and Middle school are both located closely to the west of the study intersection, and can potentially generate and attract a large amount of vehicular traffic during a short period of time. As a result, **Warrant 3 is met at the study intersection.**

3.4. Warrant 4, Pedestrian Volume

This warrant is intended application where the traffic volume on a major street is so heavy that pedestrians experience excessive delay in crossing the major street.

The need for a traffic control signal at an intersection or midblock crossing shall be considered if an engineering study finds that one of the following criteria is met:



- A. For each of any 4 hours of an average day, the plotted points representing the vehicles per hour on the major street (total of both approaches) and the corresponding pedestrians per hour crossing the major street (total of all crossings) all fall above the curve in Figure 4C-5; or
- B. For 1 hour (any four consecutive 15-minute periods) of an average day, the plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding pedestrians per hour crossing the major street (total of all crossings) falls above the curve in Figure 4C-7.

If the posted or statutory speed limit or the 85th-percentile speed on the major street exceeds 35 mph, or if the intersection lies within the built-up area of an isolated community having a population of less than 10,000, Figure 4C-6 may be used in place of Figure 4C-5 to evaluate Criterion A in Paragraph 2, and Figure 4C-8 may be used in place of Figure 4C-7 to evaluate Criterion B in Paragraph 2.

As SH 5 has a posted speed limit of 55 MPH adjacent to the study intersection, Figure 4C-6 and Figure 4C-8 were used in the analysis and are resented as below:

Figure 4. MUTCD Figure 4C-6 Warrant 4, Pedestrian Four-Hour Volume (70% Factor)

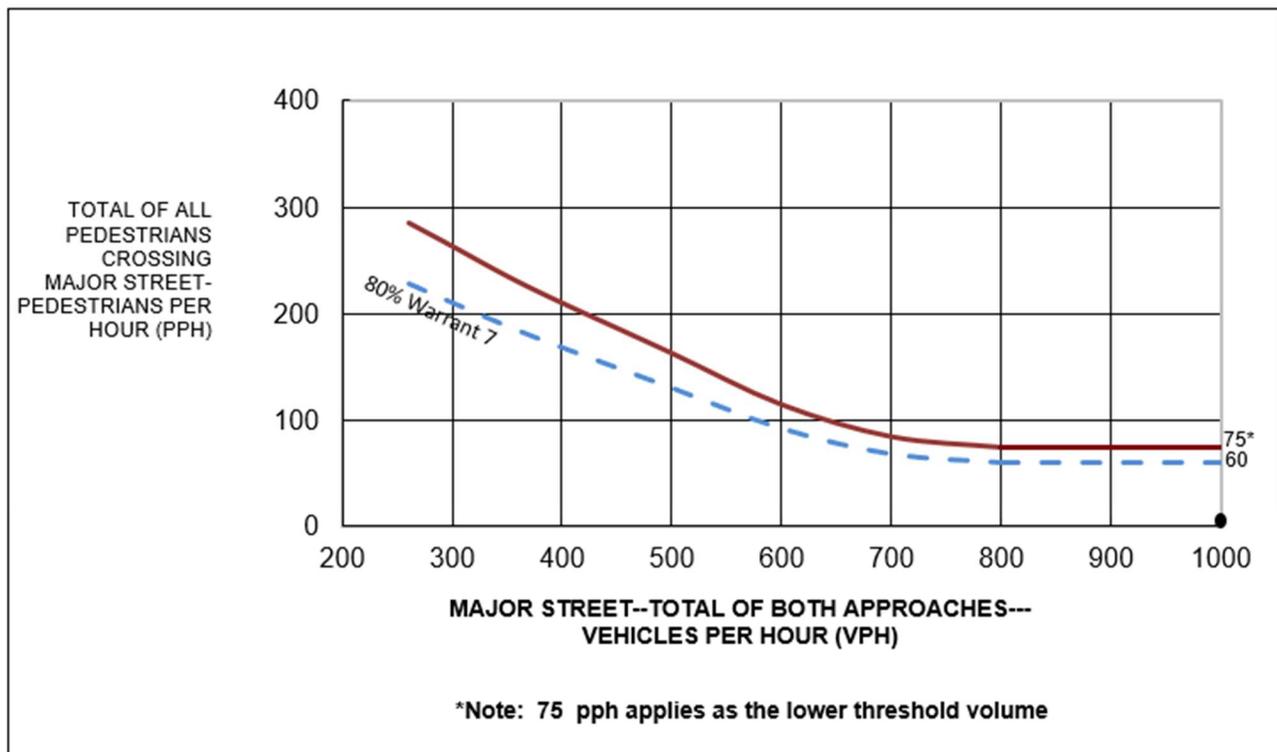
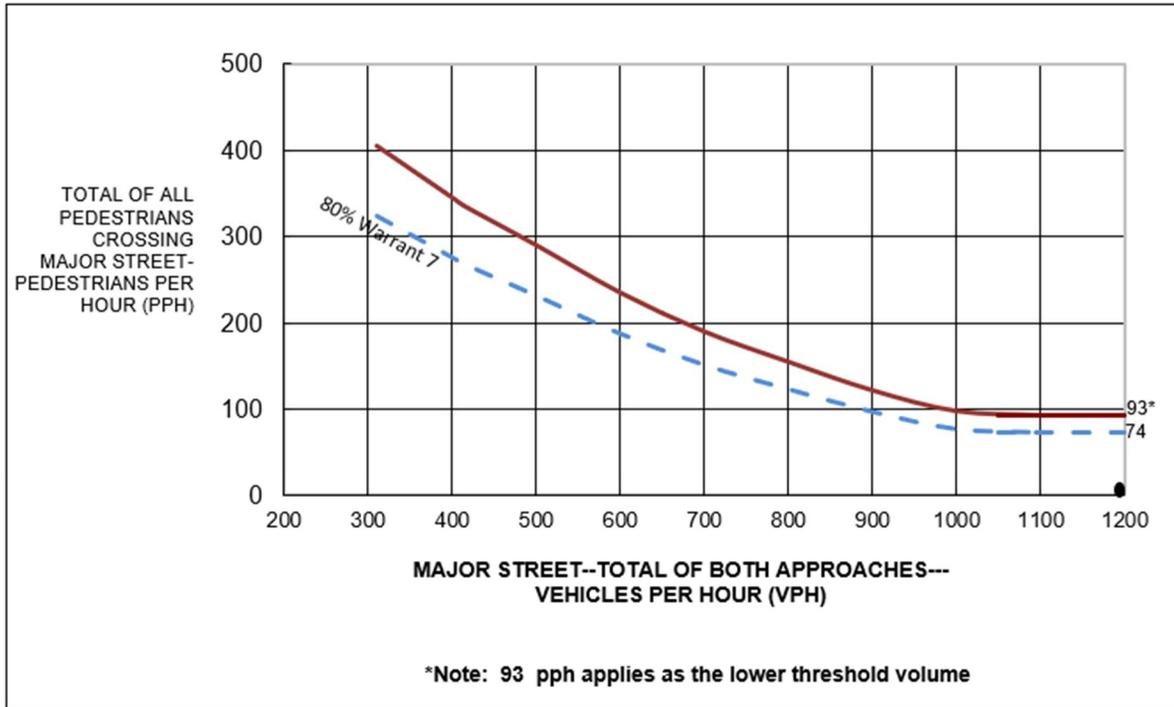




Figure 5. MUTCD Figure 4C-8 Warrant 4, Pedestrian Peak Hour (70% Factor)



According to the collected data, two pedestrians were counted for the entire 24-hour period at the study intersection. Therefore, **Warrant 4 is not met for the study intersection.**

3.5. Warrant 5, School Crossing

The School Crossing signal warrant is intended for application at locations where school children crossing the major street is the principal reason to consider installing a traffic control signal. For the purposes of this warrant, the word “school children” includes elementary through high school students.

The need for a traffic control signal shall be considered when an engineering study of the frequency and adequacy of gaps in the vehicular traffic stream as related to the number and size of groups of school children at an established school crossing across the major street shows that the number of adequate gaps in the traffic stream during the period when the school children are using the crossing is less than the number of minutes in the same period and there are a minimum of 20 school children during the highest crossing hour.

Before a decision is made to install a traffic control signal, consideration shall be given to the implementation of other remedial measures, such as warning signs and flashers, school speed zones, school crossing guards, or a grade-separated crossing.

The School Crossing signal warrant shall not be applied at locations where the distance to the nearest traffic control signal along the major street is less than 300 feet, unless the proposed traffic control signal will not restrict the progressive movement of traffic.



Per the collected traffic data, only two pedestrians were observed crossing the intersection during the 24-hour period. It is required in this warrant that there are a minimum of 20 school children crossing the major street during the highest hour. Therefore, it is determined in this study that Warrant 5 is not met due to the absence of crossing actions. However, if this intersection is to accommodate student crossing in the future, it is recommended that a gap study on the major street traffic stream be performed prior to a traffic signal warrant study.

Warrant 5 is not met for the study intersection.

3.6. Warrant 6, Coordinated Signal System

Progressive movement in a coordinated signal system sometimes necessitates installing traffic control signals at intersections where they would not otherwise be needed in order to maintain proper platooning of vehicles.

A traffic signal shall be considered if an engineering study finds that one of the following criteria is met:

- A. On a one-way street or a street that has traffic predominantly in one direction, the adjacent traffic control signals are so far apart that they do not provide the necessary degree of vehicular platooning.
- B. On a two-way street, adjacent traffic control signals do not provide the necessary degree of platooning and the proposed and adjacent traffic control signals will collectively provide a progressive operation.

The study intersection is not contained in a coordinated traffic signal system, and doesn't need to provide a necessary degree of vehicle platooning. As a result, it is determined that **Warrant 6 is not met for the study intersection.**

3.7. Warrant 7, Crash Experience

The Crash Experience signal warrant conditions are intended for application where the severity and frequency of crashes are the principal reasons to consider installing a traffic control signal.

The need for a traffic control signal shall be considered if an engineering study finds that all of the following criteria are met:

- A. Adequate trial of alternatives with satisfactory observance and enforcement has failed to reduce the crash frequency; and
- B. Five or more reported crashes, of types susceptible to correction by a traffic control signal, have occurred within a 12-month period, each crash involving personal injury or property damage apparently exceeding the applicable requirements for a reportable crash; and
- C. For each of any 8 hours of an average day, the vehicles per hour (vph) given in both of the 80 percent columns of Condition A in Table 4C-1, or the vph in both of the 80 percent columns of Condition B in Table 4C-1 exists on the major-street and the higher volume minor-street approach, respectively, to the intersection, or the volume of pedestrian traffic is not less than 80 percent of the requirements specified in the Pedestrian Volume warrant. These major-street



and minor-street volumes shall be for the same 8 hours. On the minor street, the higher volume shall not be required to be on the same approach during each of the 8 hours.

If the posted or statutory speed limit or the 85th-percentile speed on the major street exceeds 40 mph, or if the intersection lies within the built-up area of an isolated community having a population of less than 10,000, the traffic volumes in the 56 percent columns in Table 4C-1 may be used in place of the 80 percent columns. In this study, the 56 percent columns in Table 4C-1 were used as the posted speed limit on SH 5 is 55 MPH.

A crash report at the intersection for the past 12 months was obtained through TxDOT's Crash Records Information System (CRIS). According to the report, there are a total of seven reported crashes at the study intersection with one of them being a major injury case and the other six being minor injury and property damage crashes.

Since the traffic volumes do meet the minimum volumes, it is determined that **Warrant 7 is met for the study intersection.**

3.8. Warrant 8, Roadway Network

The need for a traffic control signal shall be considered if an engineering study finds that the common intersection of two or more major routes meets one or both of the following criteria:

- A. The intersection has a total existing, or immediately projected, entering volume of at least 1,000 vehicles per hour during the peak hour of a typical weekday and has 5-year projected traffic volumes, based on an engineering study, that meet one or more of Warrants 1, 2, and 3 during an average weekday; or
- B. The intersection has a total existing or immediately projected entering volume of at least 1,000 vehicles per hour for each of any 5 hours of a non-normal business day (Saturday or Sunday).

The study intersection does not serve as a common intersection of two major routes, therefore, **Warrant 8 does not apply to this study.**

3.9. Warrant 9, Intersection Near a Grade Crossing

The Intersection Near a Grade Crossing signal warrant is intended for use at a location where none of the conditions described in the other eight traffic signal warrants are met, but the proximity to the intersection of a grade crossing on an intersection approach controlled by a STOP or YIELD sign is the principal reason to consider installing a traffic control signal.

The need for a traffic control signal shall be considered if an engineering study finds that both of the following criteria are met:

- A. A grade crossing exists on an approach controlled by a STOP or YIELD sign and the center of the track nearest to the intersection is within 140 feet of the stop line or yield line on the approach; and



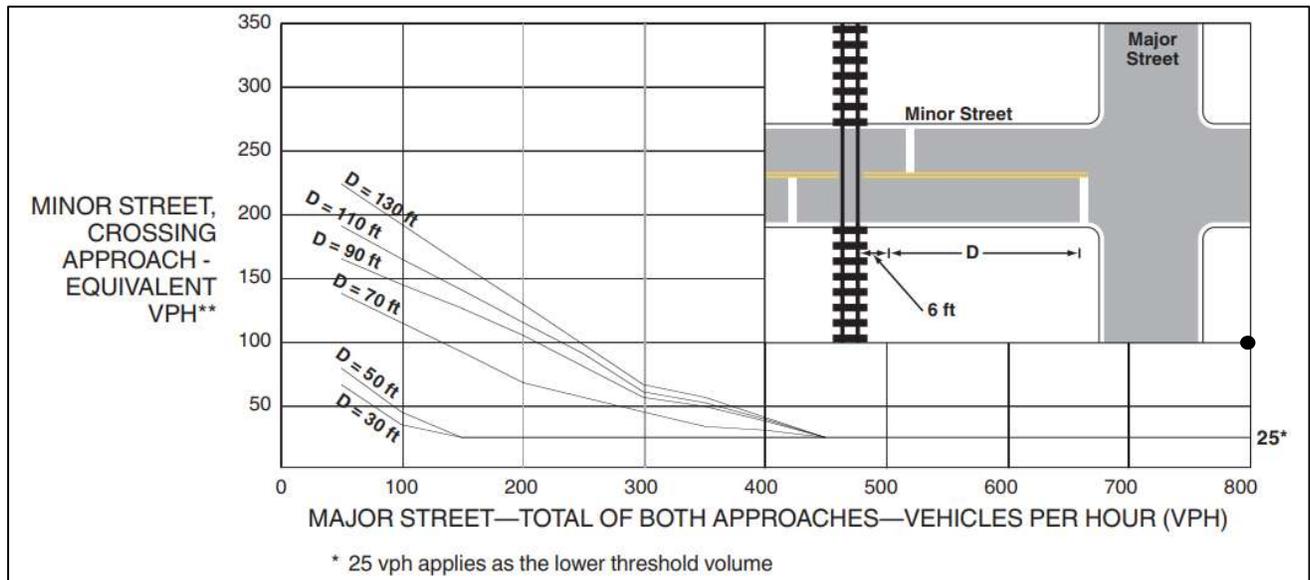
B. During the highest traffic volume hour during which rail traffic uses the crossing, the plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the minor-street approach that crosses the track (one direction only, approaching the intersection) falls above the applicable curve in Figure 4C-9 or 4C-10 for the existing combination of approach lanes over the track and the distance D, which is the clear storage distance as defined in Section 1A.13.

The distance between the center of the railroad track is approximately 180 feet from the stop line of the westbound approach at the study intersection, therefore, Criteria A of Warrant 9 is not met.

Criteria B was also analyzed based on the collected daily train volume and the arrival times at the study intersection. The major street has a total of 1059 vph for both approaches and the minor street approach that crosses the track has 118 vph during the peak hour (4:00 PM to 5:00 PM) for train traffic. Those volumes were plotted on the chart shown in Figure 6, and was determined to be above the 25 vph lower threshold of any clear storage distance D.

As a result, Warrant 9 is not met at the study intersection.

Figure 6. MUTCD Warrant 9, Intersection Near a Grade Crossing (One Approach Lane at the Track Crossing)





3.10. Results and Conclusions.

The results of the warrant analysis are summarized in Table 3. The intersection was analyzed with both roadways being a 1-lane approach in both directions, and with SH 5 being the major street and Rosamond Pkwy being the minor street.

Table 3. Signal Warrant Analysis Results

Warrant	Description	Analysis Result
1A	Eight Hour Volume – Minimum Vehicular Volume (70% & 56%)	Warrant Not Met
1B	Eight Hour Volume – Interruption of Continuous Traffic (70% & 56%)	Warrant Met
2	Four Hour Vehicular Volume	Warrant Met
3	Peak Hour	Warrant Met
4	Pedestrian Volume	Warrant Not Met
5	School Crossing	Warrant Not Met
6	Coordinated Signal System	Warrant Not Met
7	Crash Experience	Warrant Met
8	Roadway Network	Not Applicable
9	Intersection Near a RR Grade Crossing	Warrant Not Met

Of all nine signal warrants, Warrant 1B, 2, 3, and 7 are met in this study. Per the standards of Texas MUTCD, the satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal. A traffic control signal should not be installed unless an engineering study indicates that installing a traffic control signal will improve the overall safety and/or operation of the intersection, and will not seriously disrupt progressive of traffic flow.

Based on the analyses on all nine warrants, installing a traffic signal at the study intersection will improve the overall safety and/or operation of the intersection. As a result, it is determined in this study that **the installation of a traffic signal should be considered at the intersection of SH 5 & Rosamond Pkwy.**



Appendix A. Traffic Signal Warrant Analysis



Traffic Survey — Count Analysis

2014 TXMUTCD Warrants

County: Collin District: Dallas
 City: Anna Population: 20,243 Survey Date: 10/24/2023

	Name	Control	Section	85% Speed
Major	SH 5	0047	04	55 MPH
Minor	Rosamond Pkwy			30 MPH

Eight Highest Hours: Include the same 8 hours for the Major and Minor St. volumes.

Time Begins	Major St. - Both App.		Minor St. - Hi. Vol. App.		Comments:
	Veh. Total	Ped. Total	Veh. Total	Ped. Total	
4:00 PM	1,059		118		
5:00 PM	1,012		168		
6:00 PM	770		161		
7:00 AM	726		165		
3:00 PM	694		117		
8:00 AM	692		116		
12 NOON	585		73		
2:00 PM	550		83		

Warrant 1. Eight Hour Vehicular Volume

Yes No Meets 70%^c (and major-street speed exceeds 40 mph or population less than 10,000) *or* 100%^a (regardless of speed) of Condition A.
 – *or* –
 Yes No Meets 70%^c (and major-street speed exceeds 40 mph or population less than 10,000) *or* 100%^a (regardless of speed) of Condition B.
 – *or* –
 Yes No Meets 80%^b of Conditions A and B.
 – *or* –
 Yes No Meets 56%^d of Conditions A and B (and major-street speed exceeds 40 mph or population less than 10,000).

Condition A - Minimum Vehicle Volume

Number of Lanes for Moving Traffic on Each Approach		Vehicles per hour on Major St (Total of Both Approaches)				Vehicles per hour on higher-volume Minor St approach (One Direction Only)					
Major Street	Minor Street	Required				Existing	Required				Existing
		100% ^a	80% ^b	70% ^c	56% ^d		100% ^a	80% ^b	70% ^c	56% ^d	
1	1	500	400	350	280	550	150	120	105	84	73
2 or more	1	600	480	420	336		150	120	105	84	
2 or more	2 or more	600	480	420	336		200	160	140	112	
1	2 or more	500	400	350	280		200	160	140	112	

Condition B - Interruption of Continuous Traffic

Number of Lanes for Moving Traffic on Each Approach		Vehicles per hour on Major St (Total of Both Approaches)				Vehicles per hour on higher-volume Minor St approach (One Direction Only)					
Major Street	Minor Street	Required				Existing	Required				Existing
		100% ^a	80% ^b	70% ^c	56% ^d		100% ^a	80% ^b	70% ^c	56% ^d	
1	1	750	600	525	420	550	75	60	53	42	73
2 or more	1	900	720	630	504		75	60	53	42	
2 or more	2 or more	900	720	630	504		100	80	70	56	
1	2 or more	750	600	525	420		100	80	70	56	

^aBasic minimum hourly volume.

^bUsed for combination of Conditions A and B after adequate trial of other remedial measures.

^cMay be used when the major-street speed exceeds 40 mph or in a community with a population of less than 10,000.

^dMay be used for combination of Conditions A and B after adequate trial of other remedial measures when major street exceeds 40 mph or in an isolated community with a population of less than 10,000.

Warrant 2. Four Hour Volumes (70% Factor)

<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	Meets each of 4 Highest Hours (Warrant 2 — see Figure 1).
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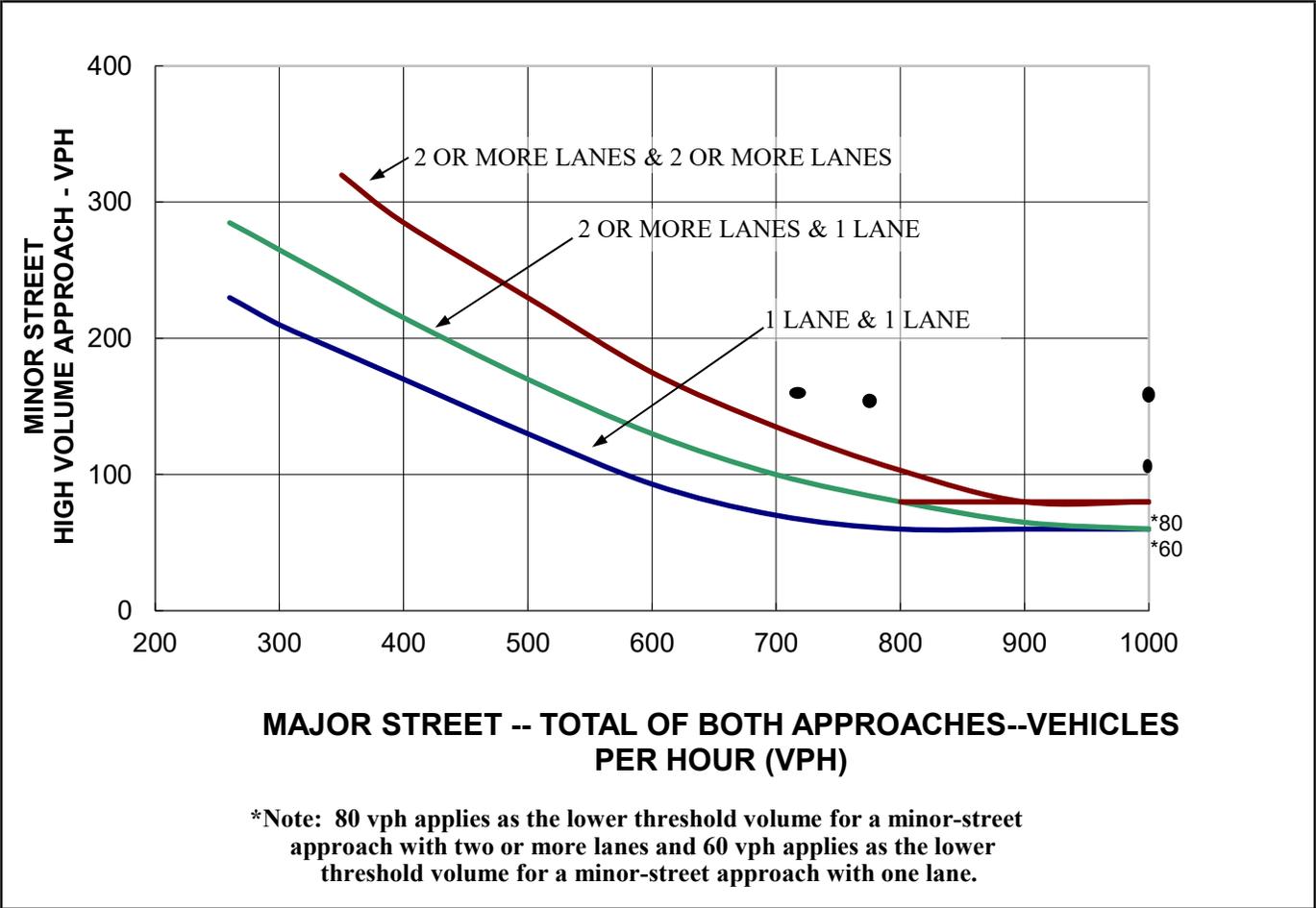


Figure 1. Four-hour volume warrant (community less than 10,000 population or above 40 MPH on major street). (Warrant 2.)

Warrant 3. Peak Hour (70% Factor)

<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No	<p>Are all of the following conditions true for any four consecutive 15 minute periods?</p> <ol style="list-style-type: none"> 1. The total stopped time delay experienced by the traffic on one minor street approach (one direction only) controlled by a stop sign equals or exceeds 4 vehicle-hours for a one-lane approach and 5 vehicle-hours for a two-lane approach, <i>and</i> 2. The volume of the same minor street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes, <i>and</i> 3. The total entering volume serviced during the hour equals or exceeds 650 vph for intersections with three approaches or 800 vph for intersections with four (or more) approaches.
<p>– <i>or</i> –</p>	
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	<p>Meets one High Hour (Warrant 3 — see Figure 2).</p>

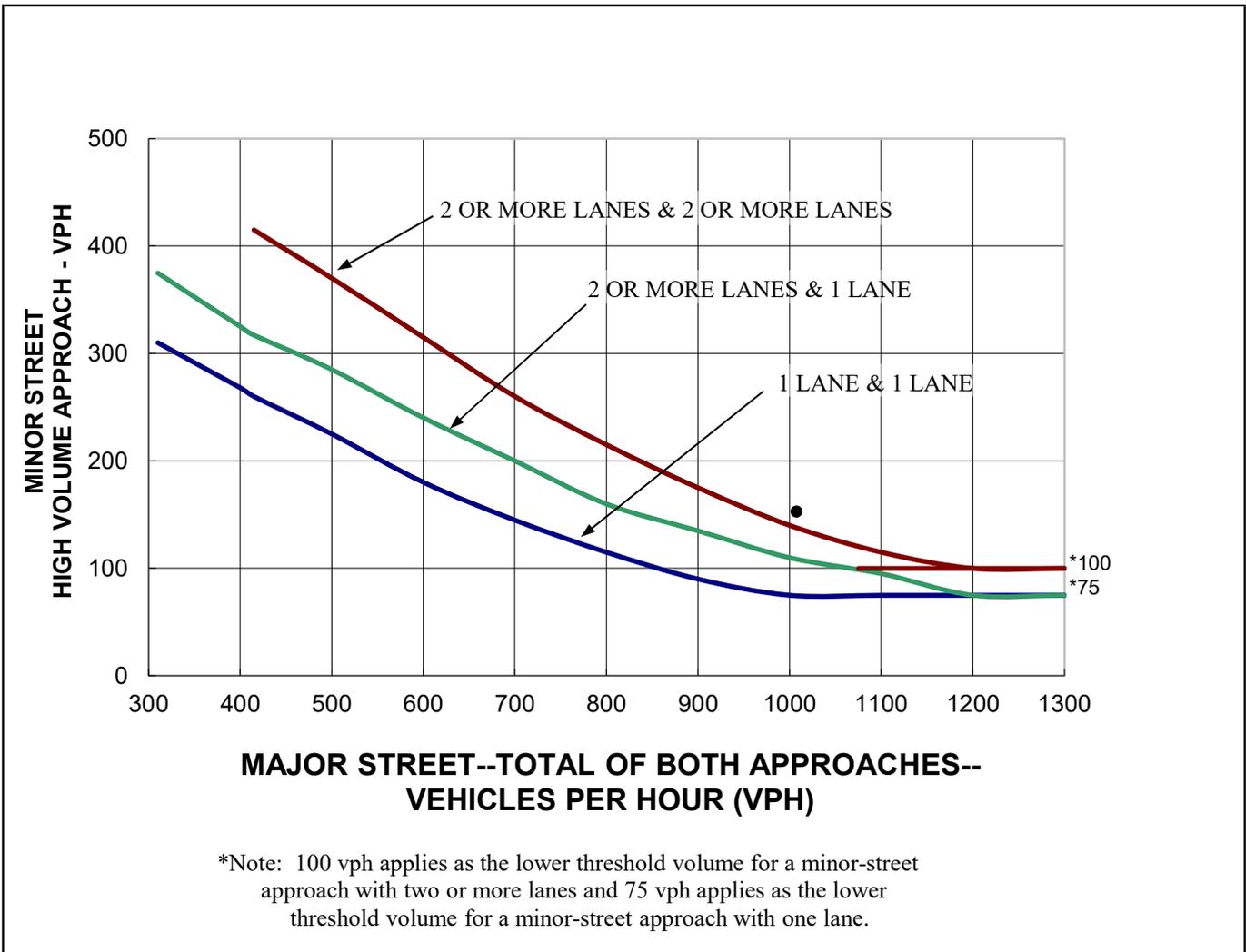


Figure 2. Peak hour volume warrant (community less than 10,000 population or above 40 MPH on major street). (Warrant 3.)

Warrant 4. Four Hour Pedestrian Volumes (70% Factor)

<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Meets each of 4 Highest Hours (Warrant4 — see Figure 3).
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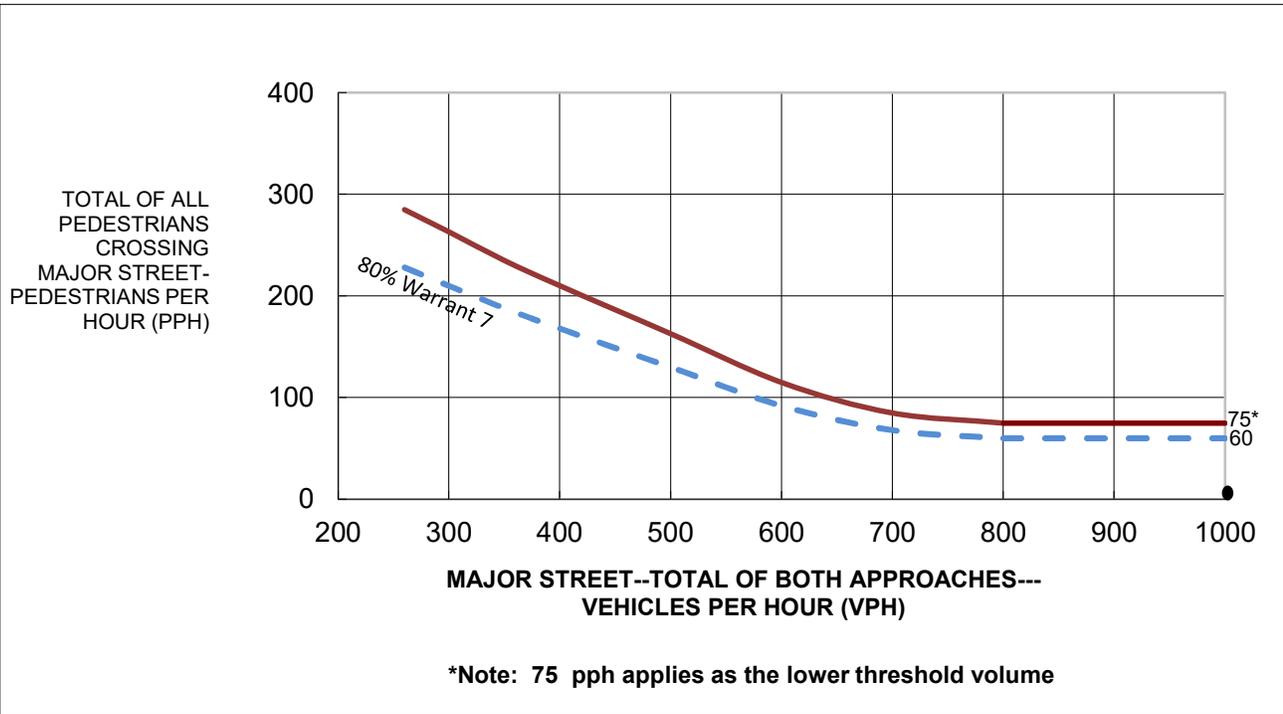


Figure 3. Four-hour pedestrian warrant (community less than 10,000 population or above 35 MPH on major street). (Warrant 4.)

Warrant 4. Peak Hour Pedestrian Volumes (70% Factor)

<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Meets Peak Hour Pedestrian (Warrant4 — see Figure 4).
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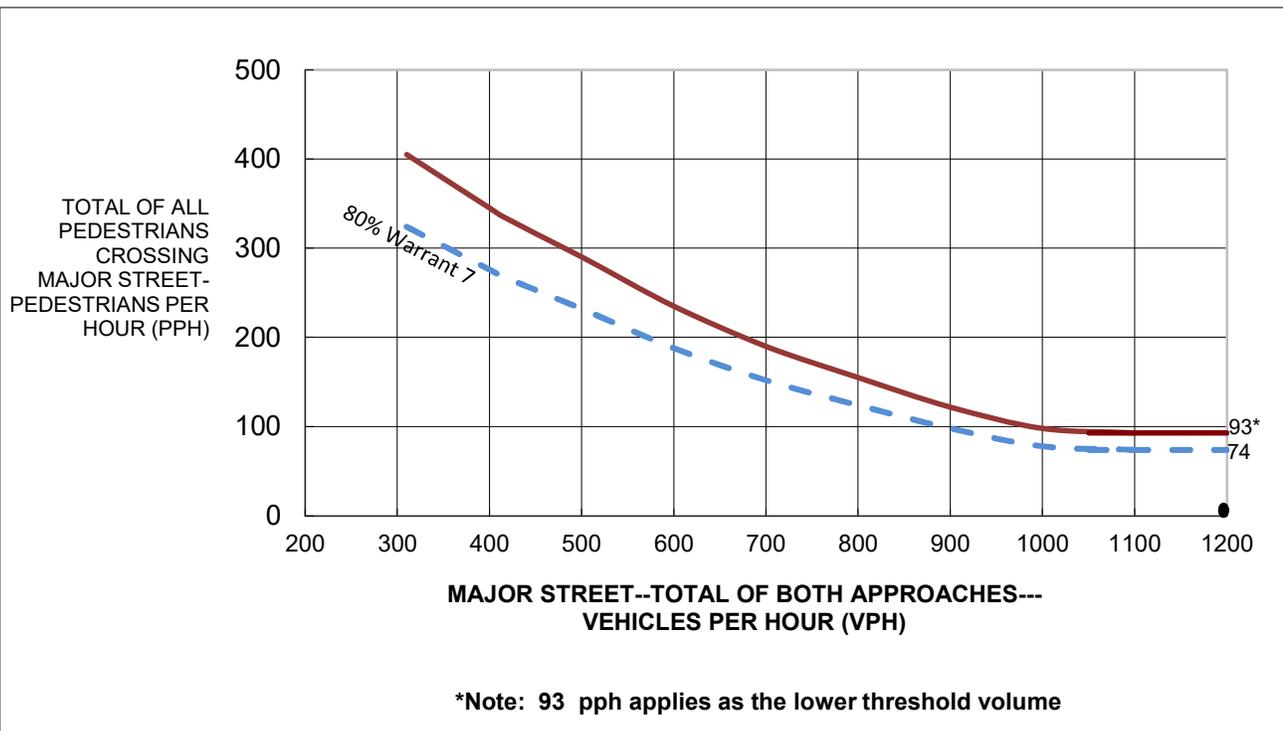


Figure 4. Peak hour pedestrian warrant (community less than 10,000 population or above 35 MPH on major street). (Warrant 4.)

Warrant 5. School Crossing

<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Is the number of adequate gaps in traffic stream during the period when the children are using the crossing less than the number of minutes in the same period? – <i>and</i> –
N/A		
<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Is there a minimum of 20 students during the highest crossing hour? – <i>and</i> –
<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Is the nearest signal located more than 300 feet away? (This warrant may be applied, if the proposed signal is less than 300 feet and does not restrict the progressive movement of traffic.)

Warrant 6. Coordinated Signal System

<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	On a one-way street or a street with traffic predominantly in one direction, are the adjacent signals far enough apart that the necessary degree of vehicle platooning does not occur? – <i>or</i> –
<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	On a two-way street, are the adjacent signals far enough apart that the necessary degree of vehicle platooning does not occur and would the proposed and adjacent traffic control signal provide a progressive operation?

Warrant 7. Crash Experience

<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	Is one of the following conditions met?: <ul style="list-style-type: none"> ◆ 80% of Condition A or Condition B in Warrant 1 ◆ 56% of Condition A or B in Warrant 1 (major-street speed exceeding 40 mph or population less than 10,000) ◆ 80 % or more of Warrant 4 met? – <i>and</i> –
<input checked="" type="checkbox"/> Yes	<input type="checkbox"/> No	Have there been 5 or more reportable crashes susceptible to correction by a traffic signal within a 12 month period?

Warrant 8. Roadway Network

<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Is the total existing, or immediately projected, entering volume on all approaches greater than 1000 vehicles for each of any 5 hours of a Saturday and/or Sunday. – <i>or</i> –
<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Is the total existing, or immediately projected, entering volume greater than 1000 vehicles for the peak hour of a typical weekday, and do the 5 year projected traffic volumes meet one or more of Warrants 1, 2, and 3 during an average weekday?

Check applicable characteristics of each route:

<u>Major Street</u>	<u>Minor Street</u>	
<input type="checkbox"/>	<input type="checkbox"/>	It is part of street or highway system that serves as the principal roadway network for through traffic flow.
<input type="checkbox"/>	<input type="checkbox"/>	It includes rural or suburban highways outside, entering, or traversing a city.
<input type="checkbox"/>	<input type="checkbox"/>	It appears as a major route on an official plan such as a major street plan in an urban area traffic and transportation study.

Remarks:

Warrant 9. Intersection Near a Grade Crossing (One Approach Lane at the Track Crossing)

<input type="checkbox"/> Yes	<input checked="" type="checkbox"/> No	Meets one High Hour (Warrant 9 — see Figure 5).
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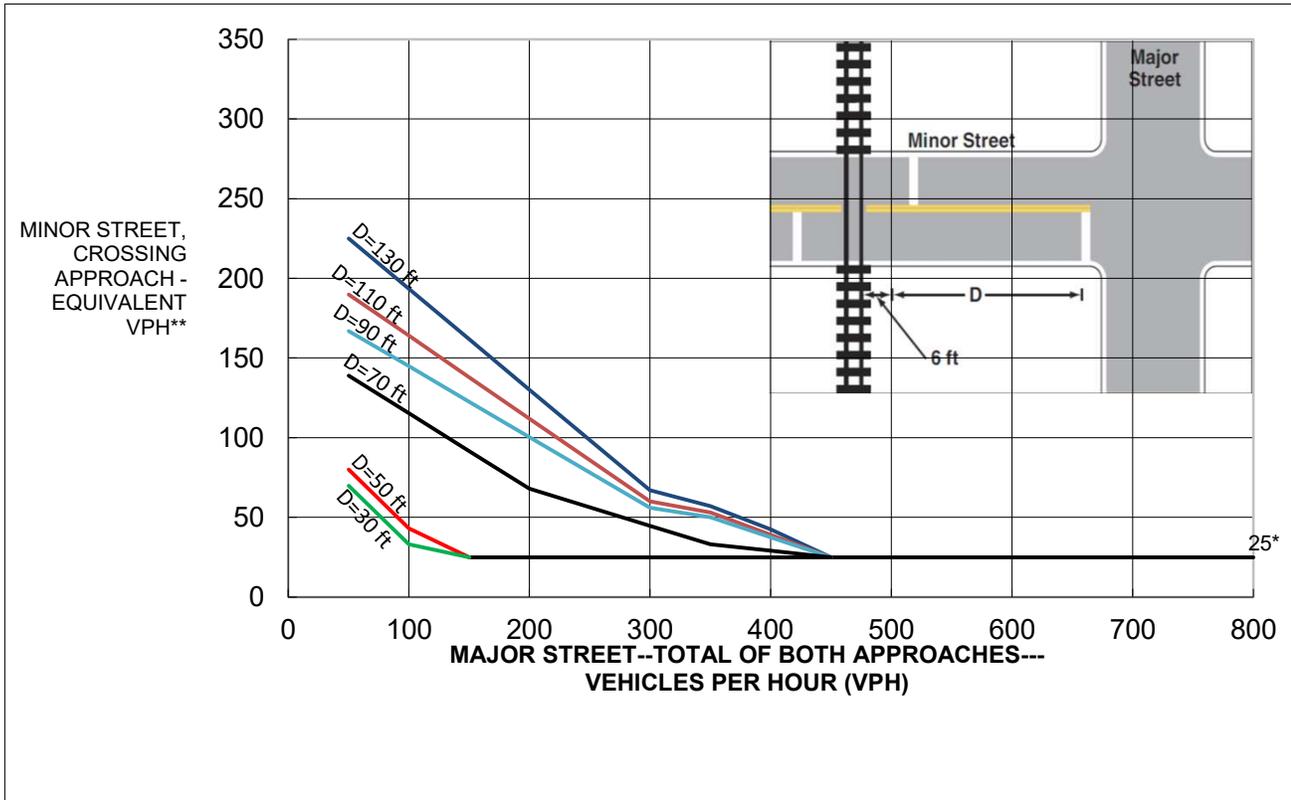


Figure 5. Railroad Grade Crossing (One Approach Lane at the Track Crossing).
(Warrant 9.)

*25 vph applies as the lower threshold volume
 ** VPH after applying the adjustment factors in Tables 4C-2, 4C-3, and/or 4C-4, if appropriate



Appendix B. 24-Hour Traffic Volume Data

Turning Movement Data

Start Time	SH 5 Southbound						ROSAMOND PKWY Westbound						SH 5 Northbound						ROSAMOND PKWY Eastbound						Int. Total	
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total		
12:00 AM	0	2	0	0	0	2	0	1	0	0	0	1	0	10	1	0	0	0	11	1	2	1	0	0	4	18
12:15 AM	0	1	0	0	0	1	0	1	0	0	0	1	0	5	1	0	0	0	6	0	3	0	0	0	3	11
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	0	0	0	0	2
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	2	1	0	0	0	0	1	3
Hourly Total	0	3	0	0	0	3	0	2	0	0	0	2	0	19	2	0	0	0	21	2	5	1	0	0	8	34
1:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
1:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	4	1	0	0	0	0	1	5
1:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	0	0	0	0	2
1:45 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	0	0	0	0	3
Hourly Total	0	1	0	0	0	1	0	0	0	0	0	0	0	8	1	0	0	0	9	1	0	0	0	0	1	11
2:00 AM	0	5	1	0	0	6	0	0	0	0	0	0	0	5	0	0	0	0	5	0	0	0	0	0	0	11
2:15 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	2
2:30 AM	0	4	0	0	0	4	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	5
2:45 AM	0	3	0	0	0	3	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	0	0	0	0	5
Hourly Total	0	13	1	0	0	14	0	0	0	0	0	0	0	9	0	0	0	0	9	0	0	0	0	0	0	23
3:00 AM	0	5	0	0	0	5	0	0	0	0	0	0	0	3	0	0	0	0	3	0	0	0	0	0	0	8
3:15 AM	0	4	0	0	0	4	0	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	5
3:30 AM	0	9	0	0	0	9	0	0	0	0	0	0	0	2	0	0	0	0	2	0	0	1	0	0	1	12
3:45 AM	0	3	0	0	0	3	0	0	0	0	0	0	1	1	0	0	0	0	2	0	0	0	0	0	0	5
Hourly Total	0	21	0	0	0	21	0	0	0	0	0	0	1	7	0	0	0	0	8	0	0	1	0	0	1	30
4:00 AM	0	2	2	0	0	4	1	1	0	0	0	2	1	1	0	0	0	0	2	1	0	0	0	0	1	9
4:15 AM	0	8	1	0	0	9	0	0	0	0	0	0	0	4	0	0	0	0	4	1	0	0	0	0	1	14
4:30 AM	0	10	1	0	0	11	1	2	0	0	0	3	0	3	1	0	0	0	4	3	0	0	0	0	3	21
4:45 AM	0	23	3	0	0	26	1	1	2	0	0	4	0	6	0	0	0	0	6	1	0	0	0	0	1	37
Hourly Total	0	43	7	0	0	50	3	4	2	0	0	9	1	14	1	0	0	0	16	6	0	0	0	0	6	81
5:00 AM	0	14	0	0	0	14	1	1	0	0	0	2	1	2	0	0	0	0	3	0	0	2	0	0	2	21
5:15 AM	0	19	4	0	0	23	5	2	1	0	0	8	2	7	0	0	0	0	9	3	0	2	0	0	5	45
5:30 AM	0	28	2	0	0	30	6	3	1	0	0	10	0	19	1	0	0	0	20	1	0	4	0	0	5	65
5:45 AM	0	48	8	0	0	56	5	3	0	0	0	8	3	28	3	0	0	0	34	7	0	1	0	0	8	106
Hourly Total	0	109	14	0	0	123	17	9	2	0	0	28	6	56	4	0	0	0	66	11	0	9	0	0	20	237
6:00 AM	2	47	7	0	0	56	7	12	1	0	0	20	8	20	3	0	0	0	31	5	0	4	0	0	9	116
6:15 AM	2	44	10	0	0	56	6	9	1	0	0	16	2	27	3	0	0	0	32	4	6	3	0	0	13	117
6:30 AM	2	67	13	0	0	82	8	9	2	0	0	19	5	22	3	0	0	0	30	9	12	4	0	0	25	156
6:45 AM	6	85	18	0	0	109	7	10	5	0	0	22	9	50	8	0	0	0	67	28	7	3	0	0	38	236
Hourly Total	12	243	48	0	0	303	28	40	9	0	0	77	24	119	17	0	0	0	160	46	25	14	0	0	85	625
7:00 AM	1	41	1	0	0	43	12	8	3	0	1	23	6	76	10	0	0	0	92	41	5	10	0	0	56	214
7:15 AM	3	89	20	0	0	112	9	15	5	0	0	29	13	45	4	0	0	0	62	26	5	11	0	0	42	245
7:30 AM	1	80	45	0	0	126	8	8	5	0	0	21	8	47	4	0	0	0	59	11	5	19	0	0	35	241

7:45 AM	2	92	32	0	0	126	2	14	1	0	0	17	49	53	4	0	0	106	5	0	27	0	0	32	281
Hourly Total	7	302	98	0	0	407	31	45	14	0	1	90	76	221	22	0	0	319	83	15	67	0	0	165	981
8:00 AM	4	113	26	0	0	143	9	8	3	0	0	20	29	87	8	0	0	124	4	4	50	0	0	58	345
8:15 AM	0	77	12	0	0	89	7	7	2	0	0	16	12	74	8	0	0	94	11	1	21	0	0	33	232
8:30 AM	0	60	10	0	0	70	6	6	2	0	0	14	11	37	7	0	0	55	3	0	7	0	0	10	149
8:45 AM	2	59	3	0	0	64	4	6	4	0	0	14	5	47	1	0	0	53	7	4	4	0	0	15	146
Hourly Total	6	309	51	0	0	366	26	27	11	0	0	64	57	245	24	0	0	326	25	9	82	0	0	116	872
9:00 AM	1	38	18	0	0	57	5	9	2	0	0	16	15	42	5	0	0	62	5	2	4	0	0	11	146
9:15 AM	0	58	4	0	0	62	7	6	1	0	0	14	5	41	6	0	0	52	5	2	13	0	0	20	148
9:30 AM	0	65	4	0	0	69	7	3	1	0	0	11	4	46	4	0	0	54	6	2	2	0	0	10	144
9:45 AM	1	71	1	0	0	73	4	1	2	0	0	7	8	42	4	0	0	54	5	0	4	0	0	9	143
Hourly Total	2	232	27	0	0	261	23	19	6	0	0	48	32	171	19	0	0	222	21	6	23	0	0	50	581
10:00 AM	1	58	4	0	0	63	10	2	1	0	0	13	3	33	2	0	0	38	3	3	4	0	0	10	124
10:15 AM	1	62	5	0	0	68	3	2	0	0	0	5	4	46	5	0	0	55	9	5	8	0	0	22	150
10:30 AM	0	52	3	0	0	55	2	2	1	0	0	5	7	36	7	0	0	50	2	1	11	0	0	14	124
10:45 AM	0	50	5	0	0	55	7	2	3	0	0	12	3	48	4	0	0	55	9	6	3	0	0	18	140
Hourly Total	2	222	17	0	0	241	22	8	5	0	0	35	17	163	18	0	0	198	23	15	26	0	0	64	538
11:00 AM	2	71	5	0	0	78	6	3	1	0	0	10	6	49	9	0	0	64	2	3	6	0	0	11	163
11:15 AM	2	68	2	0	0	72	7	3	2	0	0	12	3	48	4	0	0	55	3	2	3	0	0	8	147
11:30 AM	0	79	5	0	0	84	7	3	2	0	0	12	3	46	3	0	0	52	9	3	9	0	0	21	169
11:45 AM	0	69	8	0	0	77	6	2	0	0	0	8	1	56	4	0	0	61	2	2	12	0	0	16	162
Hourly Total	4	287	20	0	0	311	26	11	5	0	0	42	13	199	20	0	0	232	16	10	30	0	0	56	641
12:00 PM	2	74	3	0	0	79	9	0	0	0	0	9	8	61	8	0	0	77	7	4	3	0	0	14	179
12:15 PM	3	61	1	0	0	65	3	1	1	0	0	5	5	64	4	0	0	73	9	5	6	0	0	20	163
12:30 PM	0	66	11	0	0	77	4	1	2	0	0	7	11	50	5	0	0	66	5	1	9	0	0	15	165
12:45 PM	0	65	7	0	0	72	9	2	0	0	0	11	7	61	8	0	0	76	8	3	13	0	0	24	183
Hourly Total	5	266	22	0	0	293	25	4	3	0	0	32	31	236	25	0	0	292	29	13	31	0	0	73	690
1:00 PM	1	68	6	0	0	75	8	4	2	0	0	14	6	70	6	0	0	82	6	1	8	0	0	15	186
1:15 PM	0	48	4	0	0	52	5	3	1	0	0	9	4	48	7	0	0	59	11	4	5	0	0	20	140
1:30 PM	1	46	0	0	0	47	5	2	2	0	0	9	8	68	7	0	0	83	2	4	10	0	1	16	155
1:45 PM	0	42	4	0	0	46	1	3	1	0	0	5	2	55	6	0	0	63	7	2	10	0	0	19	133
Hourly Total	2	204	14	0	0	220	19	12	6	0	0	37	20	241	26	0	0	287	26	11	33	0	1	70	614
2:00 PM	0	45	8	0	0	53	0	3	0	0	0	3	4	81	6	0	0	91	13	5	4	0	0	22	169
2:15 PM	3	48	3	0	0	54	3	2	3	0	0	8	4	59	5	0	0	68	10	2	7	0	0	19	149
2:30 PM	3	83	4	0	0	90	1	2	4	0	0	7	0	60	2	0	0	62	16	0	8	0	0	24	183
2:45 PM	3	60	6	0	0	69	3	0	2	0	0	5	5	52	6	0	0	63	7	4	7	0	0	18	155
Hourly Total	9	236	21	0	0	266	7	7	9	0	0	23	13	252	19	0	0	284	46	11	26	0	0	83	656
3:00 PM	2	58	6	0	0	66	7	3	1	0	0	11	5	85	10	0	0	100	9	1	5	0	0	15	192
3:15 PM	2	72	4	0	0	78	3	2	2	0	0	7	5	62	3	0	0	70	8	3	12	0	0	23	178
3:30 PM	4	78	6	0	0	88	6	5	0	0	0	11	10	70	9	0	0	89	16	11	12	0	0	39	227
3:45 PM	1	68	23	0	0	92	6	4	2	0	0	12	9	91	11	0	0	111	19	8	13	0	0	40	255
Hourly Total	9	276	39	0	0	324	22	14	5	0	0	41	29	308	33	0	0	370	52	23	42	0	0	117	852
4:00 PM	2	101	35	0	0	138	5	8	4	0	0	17	8	90	10	0	0	108	15	4	6	0	0	25	288
4:15 PM	2	83	24	0	0	109	8	5	5	0	0	18	17	104	5	0	0	126	7	7	17	0	0	31	284
4:30 PM	1	123	27	0	0	151	11	7	2	0	0	20	18	95	10	0	0	123	9	8	17	0	0	34	328
4:45 PM	8	109	24	0	0	141	5	2	3	0	0	10	21	119	23	0	0	163	12	3	13	0	0	28	342
Hourly Total	13	416	110	0	0	539	29	22	14	0	0	65	64	408	48	0	0	520	43	22	53	0	0	118	1242
5:00 PM	3	97	25	0	0	125	7	7	3	0	0	17	23	104	13	0	0	140	10	12	32	0	0	54	336
5:15 PM	6	96	23	0	0	125	4	2	0	0	0	6	26	129	9	0	0	164	14	12	10	0	0	36	331
5:30 PM	1	101	14	0	0	116	11	8	3	0	0	22	9	116	9	0	0	134	12	11	30	0	0	53	325
5:45 PM	0	85	17	0	0	102	7	4	4	0	0	15	10	86	10	0	0	106	12	7	6	0	0	25	248
Hourly Total	10	379	79	0	0	468	29	21	10	0	0	60	68	435	41	0	0	544	48	42	78	0	0	168	1240

6:00 PM	3	75	14	0	0	92	4	4	5	0	0	13	8	111	9	0	0	128	13	12	13	0	0	38	271
6:15 PM	1	55	10	0	0	66	8	9	2	0	0	19	17	102	8	0	0	127	14	12	22	0	0	48	260
6:30 PM	1	68	14	0	0	83	6	4	3	0	0	13	16	64	7	0	0	87	17	12	13	0	0	42	225
6:45 PM	1	60	14	0	0	75	1	5	6	0	0	12	12	94	6	0	0	112	12	6	15	0	0	33	232
Hourly Total	6	258	52	0	0	316	19	22	16	0	0	57	53	371	30	0	0	454	56	42	63	0	0	161	988
7:00 PM	1	53	3	0	0	57	5	1	1	0	0	7	4	83	7	0	0	94	11	5	10	0	0	26	184
7:15 PM	2	42	5	0	0	49	3	1	0	0	0	4	5	70	9	0	0	84	7	3	17	0	0	27	164
7:30 PM	1	42	6	0	0	49	1	0	1	0	0	2	0	57	4	0	0	61	9	6	11	0	0	26	138
7:45 PM	0	36	5	0	0	41	4	1	2	0	0	7	1	57	4	0	0	62	2	2	3	0	0	7	117
Hourly Total	4	173	19	0	0	196	13	3	4	0	0	20	10	267	24	0	0	301	29	16	41	0	0	86	603
8:00 PM	1	26	5	0	0	32	3	0	1	0	0	4	4	47	5	0	0	56	4	1	5	0	0	10	102
8:15 PM	1	34	4	0	0	39	2	2	0	0	0	4	5	40	2	0	0	47	5	1	3	0	0	9	99
8:30 PM	1	27	6	0	0	34	4	0	0	0	0	4	3	30	5	0	0	38	3	1	4	0	0	8	84
8:45 PM	1	33	7	0	0	41	2	0	1	0	0	3	2	29	4	0	0	35	9	4	10	0	0	23	102
Hourly Total	4	120	22	0	0	146	11	2	2	0	0	15	14	146	16	0	0	176	21	7	22	0	0	50	387
9:00 PM	1	25	11	0	0	37	1	1	1	0	0	3	4	43	6	0	0	53	18	5	17	0	0	40	133
9:15 PM	2	24	1	0	0	27	3	0	0	0	0	3	0	43	2	0	0	45	8	1	2	0	0	11	86
9:30 PM	2	25	4	0	0	31	3	0	1	0	0	4	1	31	2	0	0	34	2	4	1	0	0	7	76
9:45 PM	1	24	1	0	0	26	0	0	0	0	0	0	0	24	6	0	0	30	1	1	1	0	0	3	59
Hourly Total	6	98	17	0	0	121	7	1	2	0	0	10	5	141	16	0	0	162	29	11	21	0	0	61	354
10:00 PM	1	21	1	0	0	23	3	0	1	0	0	4	1	19	8	0	0	28	3	2	1	0	0	6	61
10:15 PM	1	5	0	0	0	6	1	0	0	0	0	1	0	18	5	0	0	23	0	0	1	0	0	1	31
10:30 PM	0	9	0	0	0	9	1	0	0	0	0	1	0	15	5	0	0	20	1	0	0	0	0	1	31
10:45 PM	0	7	0	0	0	7	1	1	0	0	0	2	1	16	1	0	0	18	3	2	0	0	0	5	32
Hourly Total	2	42	1	0	0	45	6	1	1	0	0	8	2	68	19	0	0	89	7	4	2	0	0	13	155
11:00 PM	0	5	1	0	0	6	0	0	0	0	0	0	0	18	2	0	0	20	1	0	0	0	0	1	27
11:15 PM	1	3	0	0	0	4	0	0	0	0	0	0	1	10	1	0	0	12	0	3	0	0	0	3	19
11:30 PM	0	1	1	0	0	2	0	0	0	0	0	0	0	9	1	0	0	10	1	1	0	0	0	2	14
11:45 PM	0	1	0	0	0	1	1	0	0	0	0	1	0	3	1	0	0	4	1	1	0	0	0	2	8
Hourly Total	1	10	2	0	0	13	1	0	0	0	0	1	1	40	5	0	0	46	3	5	0	0	0	8	68
Grand Total	104	4263	681	0	0	5048	364	274	126	0	1	764	537	4144	430	0	0	5111	623	292	665	0	1	1580	12503
Approach %	2.1	84.4	13.5	0.0	-	-	47.6	35.9	16.5	0.0	-	-	10.5	81.1	8.4	0.0	-	-	39.4	18.5	42.1	0.0	-	-	-
Total %	0.8	34.1	5.4	0.0	-	40.4	2.9	2.2	1.0	0.0	-	6.1	4.3	33.1	3.4	0.0	-	40.9	5.0	2.3	5.3	0.0	-	12.6	-
Lights	102	4149	660	0	-	4911	348	266	120	0	-	734	524	4029	408	0	-	4961	600	283	636	0	-	1519	12125
% Lights	98.1	97.3	96.9	-	-	97.3	95.6	97.1	95.2	-	-	96.1	97.6	97.2	94.9	-	-	97.1	96.3	96.9	95.6	-	-	96.1	97.0
Mediums	2	80	21	0	-	103	11	6	6	0	-	23	12	83	15	0	-	110	20	9	26	0	-	55	291
% Mediums	1.9	1.9	3.1	-	-	2.0	3.0	2.2	4.8	-	-	3.0	2.2	2.0	3.5	-	-	2.2	3.2	3.1	3.9	-	-	3.5	2.3
Articulated Trucks	0	34	0	0	-	34	5	2	0	0	-	7	1	32	7	0	-	40	3	0	3	0	-	6	87
% Articulated Trucks	0.0	0.8	0.0	-	-	0.7	1.4	0.7	0.0	-	-	0.9	0.2	0.8	1.6	-	-	0.8	0.5	0.0	0.5	-	-	0.4	0.7
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-

GRAM Traffic NTX Inc.
 1120 W. Lovers Lane
 Arlington, Texas, United States 76013
 817.265.8968 isaiah@gramntx.com

Count Name: SH 5 @ ROSAMOND PKWY
 Site Code:
 Start Date: 10/24/2023
 Page No: 5

Turning Movement Peak Hour Data (7:15 AM)

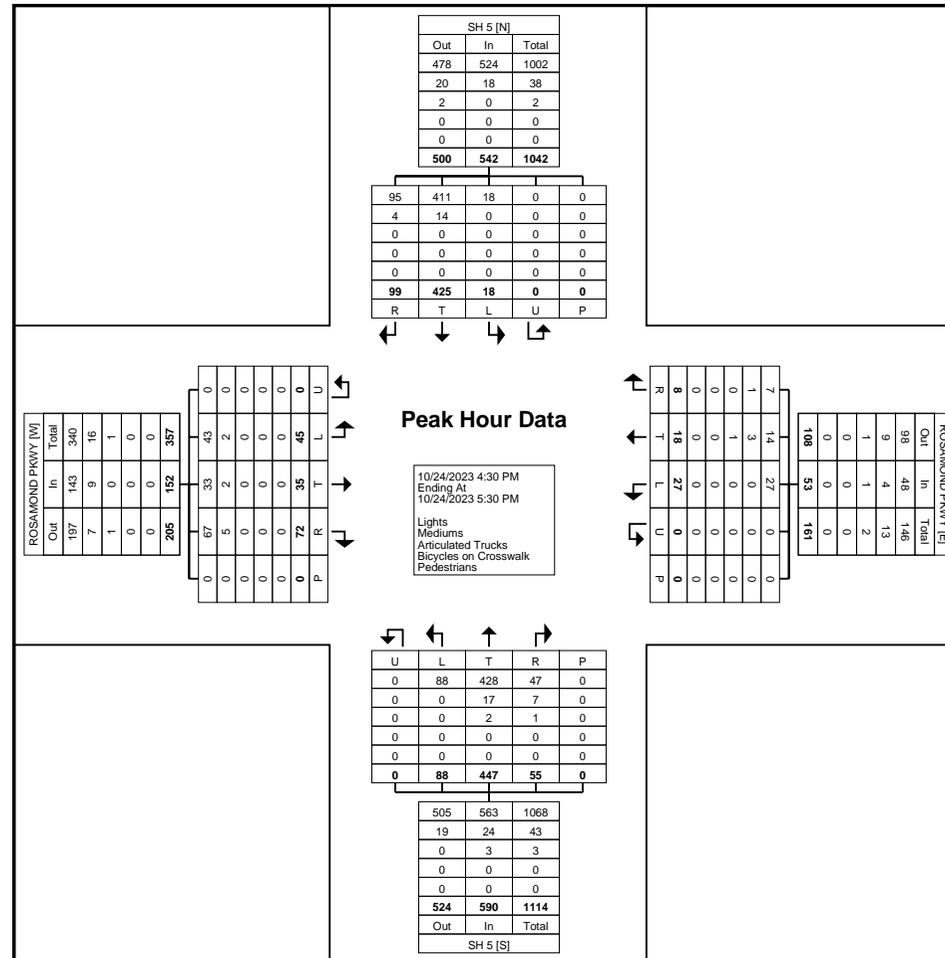
Start Time	SH 5 Southbound						ROSAMOND PKWY Westbound						SH 5 Northbound						ROSAMOND PKWY Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:15 AM	3	89	20	0	0	112	9	15	5	0	0	29	13	45	4	0	0	62	26	5	11	0	0	42	245
7:30 AM	1	80	45	0	0	126	8	8	5	0	0	21	8	47	4	0	0	59	11	5	19	0	0	35	241
7:45 AM	2	92	32	0	0	126	2	14	1	0	0	17	49	53	4	0	0	106	5	0	27	0	0	32	281
8:00 AM	4	113	26	0	0	143	9	8	3	0	0	20	29	87	8	0	0	124	4	4	50	0	0	58	345
Total	10	374	123	0	0	507	28	45	14	0	0	87	99	232	20	0	0	351	46	14	107	0	0	167	1112
Approach %	2.0	73.8	24.3	0.0	-	-	32.2	51.7	16.1	0.0	-	-	28.2	66.1	5.7	0.0	-	-	27.5	8.4	64.1	0.0	-	-	-
Total %	0.9	33.6	11.1	0.0	-	45.6	2.5	4.0	1.3	0.0	-	7.8	8.9	20.9	1.8	0.0	-	31.6	4.1	1.3	9.6	0.0	-	15.0	-
PHF	0.625	0.827	0.683	0.000	-	0.886	0.778	0.750	0.700	0.000	-	0.750	0.505	0.667	0.625	0.000	-	0.708	0.442	0.700	0.535	0.000	-	0.720	0.806
Lights	9	356	118	0	-	483	26	43	14	0	-	83	95	225	20	0	-	340	43	11	97	0	-	151	1057
% Lights	90.0	95.2	95.9	-	-	95.3	92.9	95.6	100.0	-	-	95.4	96.0	97.0	100.0	-	-	96.9	93.5	78.6	90.7	-	-	90.4	95.1
Mediums	1	17	5	0	-	23	2	2	0	0	-	4	4	7	0	0	-	11	3	3	10	0	-	16	54
% Mediums	10.0	4.5	4.1	-	-	4.5	7.1	4.4	0.0	-	-	4.6	4.0	3.0	0.0	-	-	3.1	6.5	21.4	9.3	-	-	9.6	4.9
Articulated Trucks	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	0.0	0.3	0.0	-	-	0.2	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.1
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

GRAM Traffic NTX Inc.
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Count Name: SH 5 @ ROSAMOND PKWY
 Site Code:
 Start Date: 10/24/2023
 Page No: 7

Turning Movement Peak Hour Data (4:30 PM)

Start Time	SH 5 Southbound						ROSAMOND PKWY Westbound						SH 5 Northbound						ROSAMOND PKWY Eastbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
4:30 PM	1	123	27	0	0	151	11	7	2	0	0	20	18	95	10	0	0	123	9	8	17	0	0	34	328
4:45 PM	8	109	24	0	0	141	5	2	3	0	0	10	21	119	23	0	0	163	12	3	13	0	0	28	342
5:00 PM	3	97	25	0	0	125	7	7	3	0	0	17	23	104	13	0	0	140	10	12	32	0	0	54	336
5:15 PM	6	96	23	0	0	125	4	2	0	0	0	6	26	129	9	0	0	164	14	12	10	0	0	36	331
Total	18	425	99	0	0	542	27	18	8	0	0	53	88	447	55	0	0	590	45	35	72	0	0	152	1337
Approach %	3.3	78.4	18.3	0.0	-	-	50.9	34.0	15.1	0.0	-	-	14.9	75.8	9.3	0.0	-	-	29.6	23.0	47.4	0.0	-	-	-
Total %	1.3	31.8	7.4	0.0	-	40.5	2.0	1.3	0.6	0.0	-	4.0	6.6	33.4	4.1	0.0	-	44.1	3.4	2.6	5.4	0.0	-	11.4	-
PHF	0.563	0.864	0.917	0.000	-	0.897	0.614	0.643	0.667	0.000	-	0.663	0.846	0.866	0.598	0.000	-	0.899	0.804	0.729	0.563	0.000	-	0.704	0.977
Lights	18	411	95	0	-	524	27	14	7	0	-	48	88	428	47	0	-	563	43	33	67	0	-	143	1278
% Lights	100.0	96.7	96.0	-	-	96.7	100.0	77.8	87.5	-	-	90.6	100.0	95.7	85.5	-	-	95.4	95.6	94.3	93.1	-	-	94.1	95.6
Mediums	0	14	4	0	-	18	0	3	1	0	-	4	0	17	7	0	-	24	2	2	5	0	-	9	55
% Mediums	0.0	3.3	4.0	-	-	3.3	0.0	16.7	12.5	-	-	7.5	0.0	3.8	12.7	-	-	4.1	4.4	5.7	6.9	-	-	5.9	4.1
Articulated Trucks	0	0	0	0	-	0	0	1	0	0	-	1	0	2	1	0	-	3	0	0	0	0	-	0	4
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	5.6	0.0	-	-	1.9	0.0	0.4	1.8	-	-	0.5	0.0	0.0	0.0	-	-	0.0	0.3
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Turning Movement Peak Hour Data Plot (4:30 PM)



Appendix C. Crash Records from CRIS for the Past 12 Months

Case ID	Crash Date	Crash Number	Crash Severity	Street Name	Intersecting Street Name	Manner of Collision
22195758	12/25/2022	2022640299	N - NOT INJURED	SH0005	W ROSAMOND PKWY	ONE MOTOR VEHICLE - TURNING LEFT
23032347	2/23/2023	2023091353	B - SUSPECTED MINOR INJURY	SH0005	W ROSAMOND PKWY	ANGLE - BOTH GOING STRAIGHT
23063101	4/13/2023	2023188420	N - NOT INJURED	SH0005	W ROSAMOND PKWY	ANGLE - ONE STRAIGHT-ONE LEFT TURN
23063848	4/14/2023	2023170041	A - SUSPECTED SERIOUS INJURY	SH0005	W ROSAMOND PKWY	ANGLE - BOTH GOING STRAIGHT
23186356	10/3/2023	2023481583	B - SUSPECTED MINOR INJURY	SH0005	W ROSAMOND PKWY	ANGLE - BOTH GOING STRAIGHT
23193444	10/13/2023	2023499265	N - NOT INJURED	SH0005	W ROSAMOND PKWY	ANGLE - ONE STRAIGHT-ONE LEFT TURN
23195503	10/17/2023	2023516464	N - NOT INJURED	SH0005	W ROSAMOND PKWY	ANGLE - BOTH GOING STRAIGHT